

Australasian Structural Engineering Conference

Engineering Resilience

Australasian Structural Engineering Conference

9-10 November 2022



aseconference.org.au



Critical Reinforcement Design and Detailing for Resilience and Preservation of Concrete Structures © SRIA 2022

Speaker



Eric Lume

BE MIE Aust CPEng (Ret) National Engineer at the Steel Reinforcement Institute of Australia.

Prior to the last seven and a half years with the SRIA, Eric's extensive industry experience includes a recent engagement with a Consulting Engineering firm in Christchurch, NZ assisting with rebuilding earthquake damaged structures, 5 years as Senior Lecturer at the University of Wollongong, 15 years with Cement Concrete and Aggregates Australia, and some 13 years in the Consulting Engineering Profession. He is also on a number of Australian Standard's Committees.

Web Site: sria.com.au



Welcome to SRIA

Steel Reinforcement Institute of Australia

The Steel Reinforcement Institute of Australia is Australia's leading non-profit institute for reinforcing steel, providing the hub for knowledge, industry linkage and support.

- Supports Australian capability & quality
- Offers practical solutions to the Australian building industry
- Educates industry
- Disseminates steel reinforcement knowledge via regular publications, lectures, seminars, research programs and tours
- Primarily funded by the vast majority of the processors of steel reinforcement used in Australian construction
- Supported by the founding Australian mill (supplier) members & associate members





Resilience

Resilience encapsulates our ability to not only survive disasters and extreme events such as bushfires, floods and earthquakes, but to also recover more quickly from them, with reduced impact on not only peoples' lives, but also in many cases, their livelihoods.



Royal National Park, Sydney

5

Maribyrnong, Melbourne

Newcastle Earthquake, 1989





Over the next 40 years, natural disasters will cost Australia \$1.2 trillion Currently \$38 billion annually, rising to \$73 billion by 2060

Cost of earthquakes relatively low due to low to moderate seismicity in Australia



Earthquake Frequency in Australia

- Low to moderate seismicity.
- Earthquakes are a regular occurrence.
- Often occur in isolated areas, but
- All capital cities are expected to get a Newcastle type earthquake at some point (except Darwin & Hobart).
- Sydney in top 10 financial risks worldwide.
- On average Australia <u>will</u> experience:
 - 1 shallow earthquake Magnitude 6.0 or more once every 10 years (Christchurch was M6.2)
 - I shallow earthquake ≥ M5 every 2 years
 - ₂ ▶ 2 M5 events every year

HAZARD NOTE Bushfire & Natural Hazards CRC Issue 112 February 2022



that a moderate earthquake in	satemically vulnerable forms of construction	concrete and/or unreinforced masonry.
their top-10 financial risks, there	In Australia - unwinforced masonry buildings	The evidence-based recommendations
tion in the Australian construction	and low-ductility reinforced concrete	that this project developed for unwimforced
at design for earthquakes is a	buildings - and cost-effective techniques to	masonry buildings were implemented by the
money due to the low likelihood	mitigate damage. This evidence base enables	York Shire Council in Western Australia. York
earthquake in Australia. As the	building owners and government to make	has many heritage masonry buildings on the
2021 earthquake in Victoria	cost-effective decisions about strengthening	WA and national heritage registers that are
iss like Helbourne are not immune	buildings against earthquake damage.	vulnerable to earthquake, with the heritage
ike damage. This research	Researchers not only examined the impacts	value of the area significantly contributing
risk and economic-loss models	of building damage, but importantly also	to the local economy. A historic museum in
akes, which allow authorities	accounted for latalities/injuries, business	York is now being used as a demonstration
cost-boneilt analyses for	Interruption and heritage structure impacts.	of effective examples of strengthening
by justifiable setunic-strengthening	These lindings are also relevant to the impact	against earthquakes, to develop skills in the
its for existing buildings.	of earthquakes on other infrastructure	local construction and building industries.
ROUND	the rest-disater anemetry management	The two most voltarishin hubbles types
as have only been recoonised	logistics and community recovery needs	that contribute disproportionately to

Summary

Although the international reinsurance industry recognises that a moderate earthquake in Sydney is in their top 10 financial risks, there is a perception in the Australian construction industry that design for earthquakes is a poor use of money due to the low likelihood of a strong earthquake in Australia. As the September 2021 earthquake in Victoria showed, cities like Melbourne are not immune to earthquake damage.

Problem with Low to Moderate Seismicity

Ratio of Rare Event (2500 year) to Design Event (500 year)

- Seismic risk in Australia is considered to be low to moderate (bottom curve in graph)
- Australia is low risk but high consequence country in term of earthquake damage
- 1:500 cf 1:2,500 event PROBLEM is peak ground acceleration nearly 4 times greater for Australia
- ➡ 1:2,500 Most buildings would not survive
- Proposed 2500 year design requirement for CBD areas?
- Blanket minimum design requirement across Australia?





Australia Experiences Earthquakes

1 January to 18 April 2022

 Magnitude: 1.9 to 4.8 (283 events)



Meckering and Newcastle Earthquakes

Fault line scarp from the Meckering, Western Australia 1968, Magnitude 6.9 earthquake.







Influence of the Newcastle earthquake Magnitude 5.6



Damage to the Newcastle Workers Club, which was subsequently demolished and rebuilt (Photo Courtesy Newcastle Library).



The New Royal Adelaide Hospital

Earthquake Design Hazard

Parra Fault

- ➡ Length 54 km
- Proximity to fault raised concern about vertical ground acceleration
- Distance to site 2 km
- Max. predicted earthquake Magnitude 7.5





Christchurch Earthquake, 2011 – M6.2

- ♦ \$55 billion loss
- Population 370,000



Christchurch Art Gallery Bookstore during 2011 earthquake

Christchurch CBD: 90% demolished (over 800 buildings)



Resilience of Reinforced Concrete Buildings

FM Global Annual Report 2021

- Resilience is a choice by Clients
- Works with clients to improve resilience, and
- Minimise potential losses from natural disasters
- Produced Worldwide Earthquake Map
- Bldg. Fires considered most significant risk exposure
- Strategies to mitigate fire risk include:
 - Retrofitting of solid (concrete) floors
 - Replace combustible walls with fire-retardant ones
- Strategies also work for flooding

13

Solid walling types unaffected by water



Benefits of Reinforced Concrete are not New

A Paper read before the Queensland Institute of Engineers, Inc. - June 17 1913 by European Engineer L. Messy

- Highlights the rapid acceptance and widespread use of reinforced concrete.
- Highlights the many benefits of reinforced concrete.

"In view of the present enormous developments of reinforced concrete in Europe, as well as in America...... there is no town or country, there is even no large building, without more or less use and utilisation of reinforced concrete.....

The main features of reinforced concrete are: (1) fireproof, (2) antproof, (3) waterproof, (4) easy to build, (5) no skilled labour needed, (6) lowest cost of insurance, (7) substantiality, (8) light construction, (9) good, aesthetic, and attractive appearance, (10) impermeable, (11) unaffected by hot or cold weather, (12) or by sea water, (13) durability, (14) soundproof, (15) decreased maintenance, &c., &c."



Benefits of Reinforced Concrete

A Paper read before the Queensland Institute of Engineers, Inc. - June 17 1913

 Concludes that when properly designed and constructed, reinforced concrete is probably the most valuable material for use in buildings (and structures).

"In conclusion, I may be justified in quoting a resolution of the Congress of Civil Engineers in London which says: "Reinforced concrete combines the structural qualities of steel and timber with the durability of good masonry. It is subject to no form of deterioration which cannot be avoided by reasonable precautions. It is free from many of the limitations surrounding the use of masonry in masses, because of the greater latitude it affords in the design and execution of structures. It often yields the best and most economical solution, and in some cases the only practical solution, of the most difficult problems. When properly designed and executed it is therefore among the most valuable, if not the most valuable material now available for use in connection with building and hydraulic works of all kinds."



Lessons Learnt from Christchurch Earthquake

Incorporated into AS 3600 in 2018

- Direct load paths
- Design and connection of diaphragms
- Ductility of walls
- Boundary elements to walls
- Non structural parts and components
- Ceilings and services
- Seismic movement joints/gaps
- Structural integrity reinforcement
- Anchorage of fitments













Structural Integrity Reinforcement for Slabs

Increases resistance of structural system to progressive collapse

Simple Reinforcement Detailing
Improves Life Safety





Common to assess punching shear based on only 3 sides of column



Structural Integrity Reinforcement for Slabs

Providing Structural Integrity Reinforcement – Improves Life Safety



Remains of car park floor – Old Newcastle Workers Club NSW Brittle failure with no structural integrity reinforcement

→ caused progressive collapse

(Photo courtesy Cultural Collections, The University of Newcastle, Australia)



Punching shear failure Collapse prevented by Structural Integrity Reinforcement

Hotel Grand Chancellor, Christchurch, NZ



Minimum Structural Integrity Reinforcement

AS 3600 Response

Clause 9.2.2 Minimum structural integrity reinforcement

The summation of the area of bottom reinforcement connecting the slab, drop panel, or slab band to the column or column capital on all faces of the periphery of a column or column capital shall not be less than, $2 N^*$

$$A_{\rm s.min} = \frac{2N}{\phi f_{\rm sy}}$$

Integrity reinforcement shall not be required if there are beams containing shear reinforcement and with at least two bottom bars continuous through the joint in all spans framing into the column.













Structural Integrity Reinforcement for Slabs

Distribute evenly on all faces of the column







Technical Enquiry – Post-tensioned slab detailing

Clause 9.2.2 of AS 3600 (paragraph 2) states that: "Integrity reinforcement shall not be required if there are beams containing shear reinforcement and with at least two bottom bars continuous through the joint in all spans framing into the column" – Intended for beams, not slab bands

- ➡ Relates to beam Clause 8.3.1.1 (i) and (ii) which requires minimum of two bars
- Note that 2N12 bars can be taken to contribute to $A_{s,min}$ required







Example – Builder Technical Enquiry

Builder had concerns that a PT slab band arrangement did not have the minimum integrity reinforcement over column

- No bottom reinforcement over column
- Therefore, NO structural integrity reinforcement provided
- Do not misinterpret requirements of Clause 9.2.2 by taking area of post-tensioning tendons as satisfying the minimum area of structural integrity reinforcement – structural integrity reinforcement must be in the bottom of the slab band.



SRIA Technical Enquiry



Guidance Provided in Commentary Clause C9.2.2

Simple Reinforcement Detailing - Improves Life Safety

States that:

"Post-tensioning tendons within the column head in the top surface which would normally be considered to contribute to robustness based on a catenary model are not considered to contribute to the post punching shear failure capacity."

Remains of post-tensioned car park floor – Christchurch







Prefabricated beam cages can be joined at column locations

- Satisfactory for ordinary moment-resisting frames (OMRF)
- Placement of splice bars at joints avoids interference



BASIC CAGE ARRANGEMENT

Figure 13.8 Reinforcement Detailing Handbook



Figure 13.8 Reinforcement Detailing Handbook



 \checkmark

Detailing of Beams in IMRF's

Minimum Splice and Fitment requirements for IMRF

S1 Region Fitment spacing Clause 14.5.2.2 Max. $\leq 0.25d_{o}$ $8d_{b}$ $24d_{f}$ 300 mmS2 Region Fitment spacing Max. $\leq 0.5D$ 300 mmFigure 13.10 Reinforcement Detailing Handbook

strength ≥ 33% -ve moment strength at face of either by at least 2 closed column joint. Moment strength ≥ 20% maximum moment fitments at each splice Extend L_{svt} straight or strength at face of either column joint. ≥ 33% total -ve Note: position of splice to 0.5 L_{sy,t} where cogged moment tensile reinforcement required at support shall be be determined by designer into core extended D beyond the point of contraflexure, as per Clause to avoid position of 8.1.10.3. of AS 3600. Minimum of 2 bars, continuous top maximum moment and bottom Splice* Colum -d Splice* core Closed Closed Closed 50 max Fitments Fitments Fitments 50 Fitments S2 S1 S1 S1 max L Terminate all required top and bottom Plastic hinge Plastic hinge bars at the far face of the column core, locations locations providing minimum distance L.,, for tension as per Section 13.1 of AS3600 211 Engineer must provide dimensions L1, Beam B Beam A S1,S2, fitment and closed fitment spacing, anchorage length, cut-off points of Exterior Interior discontinuous bars and L Column Beam-column Beamjoint column joint

Longitudinal reinforcement, top and bottom +ve moment

*Lap splices to be confined

Steel Reinforcement Institute of Australia



Closed Fitments

Clause 1.6.3.11 Closed fitment

External or internal fitment that forms a continuous perimeter around a concrete element with the ends of the fitment anchored into the concrete using a minimum of 135° hooks around a longitudinal bar.



Figure 9.1 from Reinforcement Detailing Handbook Standardised Bending Shapes for Reinforcement





Anchorage of Fitments

Clause 8.3.2.4 Anchorage of shear reinforcement

The anchorage of shear reinforcement (fitments) transverse to the longitudinal flexural reinforcement shall be achieved by:

- a hook or cog complying with Clause 13.1.2.7, or
- by welding of the fitment to a longitudinal bar, or
- by a welded splice, or
- by lapped splices (some Engineers allowing this!).

NOTE:

- Site welding not recommended (generally poor quality).
- Difficult achieving sufficient weld to a longitudinal bar.
- Lapped splices (in fitments) intended only for deep infrastructure type beams to allow fabrication
- ➡ AS 5100.5 requires hook at ends of lapped bars if near concrete surface.





Anchorage of Fitments

AS 3600 Commentary Published 25 March 2022

Provides additional background information and clarification of Clauses.



⁽c) Satisfactorv

Open fitments shown in Figure C8.3.2.4(B)(b) 'do not provide confinement for the concrete in the compression zone and are undesirable in heavily reinforced beams where confinement of the compressive concrete may be required to improve ductility of the member.'



Figure C8.3.2.4(B) — Incorrect, undesirable and satisfactory fitment anchorages (Ref. 37)



Interpretation of Standard

Clause 8.3.2.4 Anchorage of shear reinforcement

Hooks to anchor shear fitments

- Do not form a closed fitment refer Clause 1.6.3.12
- Torsional reinforcement requires closed fitments Clause 8.3.3(a)
- Clause 8.3.1.6 "Compressive reinforcement required for strength in beams shall be adequately restrained by fitments in accordance with Clause 10.7.4"
- ACI typically requires closing tie at top



29

Transfer Beam 1200 Deep x 1200 wide



Figure R9.7.7.1 from ACI 318M-19



Fitments and Bars need to be Adequately Anchored

Provide closed fitments anchored in confined core of beam/column Anchor beam bars in confined column core

Why? At about 1.5% drift, the cover concrete will typically be lost



Bottom bars not anchored in the confined region of the column 30

Failure of a beam column joint at Copthorne Hotel, Christchurch 2011 (Images courtesy of Peter McBean Wallbridge and Gilbert)





Spacing of Fitments

Clause 8.3.2.2 Spacing of Shear and Torsional Reinforcement

"In members not greater than 1.2 metres in depth, the maximum longitudinal spacing shall not exceed the lesser of 300 mm and 0.5*D*; otherwise, the longitudinal spacing shall not exceed 600 mm."

Clause 8.2.1.6 Requirements for transverse shear reinforcement

Transverse shear reinforcement shall be provided in all regions where:

- $V^* > \phi(V_{uc} + P_v)$; or
- → $T^* > 0.25 \phi T_{cr}$: or
- → The overall depth of the member $D \ge 750$ mm





Spacing of Fitments

Where no shear reinforcement is required and D < 750 mm

 Provide stirrups to allow assembly and support of reinforcing bars (and unexpected loads or overloads).

Recommend maximum 600 mm spacing

Keep same type as shear reinforcement to simplify fabrication

Minimise number of different spacings along beam

Commentary Clause R9.6.3.1 of ACI 318M-19 also states:

"For repeated loading of beams, the possibility of inclined diagonal tension cracks forming at stresses appreciably smaller than under static loading should be taken into account in design. In these instances, use of at least the minimum shear reinforcement expressed by 9.6.3.4 is recommended even though tests or calculations based on static loads show that shear

³² reinforcement is not required."





Download for free at sria.com.au



TECHNICAL NOTE November 2022



The Steel Reinforcement Institute of Australia is a national non-nmfit general guidance only, and in no way replaces the services of professional consultants on particular projects no legal liability can be accepted for its use.

•

FITMENTS

Fitments are defined in Clause 1.6.3.42 of AS 3600 (2018)¹ as a "Unit of reinforcement commonly used to restrain from buckling the longitudinal reinforcing bars in beams, columns and piles; carry shear, torsion and diagonal tension; act as hangers for longitudinal reinforcement; or provide confinement to the core concrete." A typical fitment is shown in Figure 1. While AS 3600 has always adopted the terminology 'fitment', they are commonly referred to as either ties, stirrups, ligatures or helix. Note that the term 'tie' was redefined in AS 3600 (2009)² as a "tension member in a strutand-tie model", and the term helix refers to helical reinforcement, which was introduced in AS 3600 (2009), with the same definition now included in Clause 1.6.3.50 of AS 3600 (2018), "reinforcement that is wound in a helical fashion around the main longitudinal reinforcing bars in a column or pile restraining them from buckling and to carry shear. torsion and diagonal tension or around tendons at an anchorage to resist bursting action effects."



Figure 1

Typically fitments will be closed fitments as defined in Clause 1.6.3.12 of AS 3600 (2018), "A unit or multiple units of reinforcement used as an external or internal fitment that form a continuous perimeter around a concrete element with the ends of the fitment anchored into the concrete using a minimum of 135° hooks around a longitudinal bar." While a closed fitment would typically resemble the single unit or shape shown in Figure 1, the reference to multiple units is to allow for the various options covered in Clause 8.3.2.4 of AS 3600 for the anchorage of shear reinforcement These include anchorage by a "hook or cog complying with

Clause 13.1.2.7 or by welding of the fitment to a longitudinal Typical closed fitment bar or by a welded splice, or by lapped splices."

Page 1



Resilience Starts with Quality Materials

Third Party/Independent Certification - ACRS or Equivalent Should be specified on every project and obtained to guarantee quality

ACRS Mill Certificate Example



<text><text><text><text><text><text><section-header><section-header><section-header>

ACRS Processor Certificate Example

 Required by Purchasers to prove quality reinforcement delivered to site

SRIA members must have ACRS Certification

Processed steel reinforcing materials may only be relied upon as having the benefit of ACRS Product Scheme certification when manufactured by ACRS certified mills.



Reinforced Concrete has a Long History

First reinforced concrete structure in Australia

Johnstons Creek Sewer Aqueduct Annandale, Sydney (started 1895)



Still in us today





Preserving Historic Structures

Smith and Waddington's Limited Factory, Sydney, circa 1927



Construction circa 1927 (Innes-Bell Concrete Construction Booklet)

Re-purposed into Luxury Apartments Rhodes House, 1993







SRIA Guides to Assist Designers, Certifiers and Builders

- Covers design and detailing requirements for Australia
- Good detailing practices covered
- Checklists included
- Free Pdf download at SRIA.COM.AU



Second Edition 2016



Conclusions

- SRIA is here to help
- Cost of natural disasters set to increase
- Resilience is a choice
- Benefits of reinforced concrete improve resilience
- Proper design and construction is essential
- AS 3600 (2018) has addressed many of the lessons learnt regarding earthquake performance
- Changes will ensure future buildings are more resilient
- Preservation of buildings/structures improves sustainability
- Resources available to assist Engineers (SRIA.COM.AU)







SRIA website provides links to industry - sria.com.au

















Mesh & Bar Pty Ltd









SRIA website provides links to suppliers - sria.com.au









Stay Connected

SUBSCRIBE

Register on SRIA web site

- Newsletter
- Latest Publications
- Seminars and Conferences
- Industry News

Free bar and mesh gauges (order on web site)



NEWS AND INDUSTRY UPDATES FULL NAME EMAIL ADDRESS Enter Word Verification in box below rvjtmy

STAY CONNECTED WITH THE LATEST



Link on web







