



A Review of Recent Australian Bond Test Results and the New Stress Development Design Rules of AS 3600–2009

Scott Munter – Steel Reinforcement Institute of Australia

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Recent Australian Bond Test Results

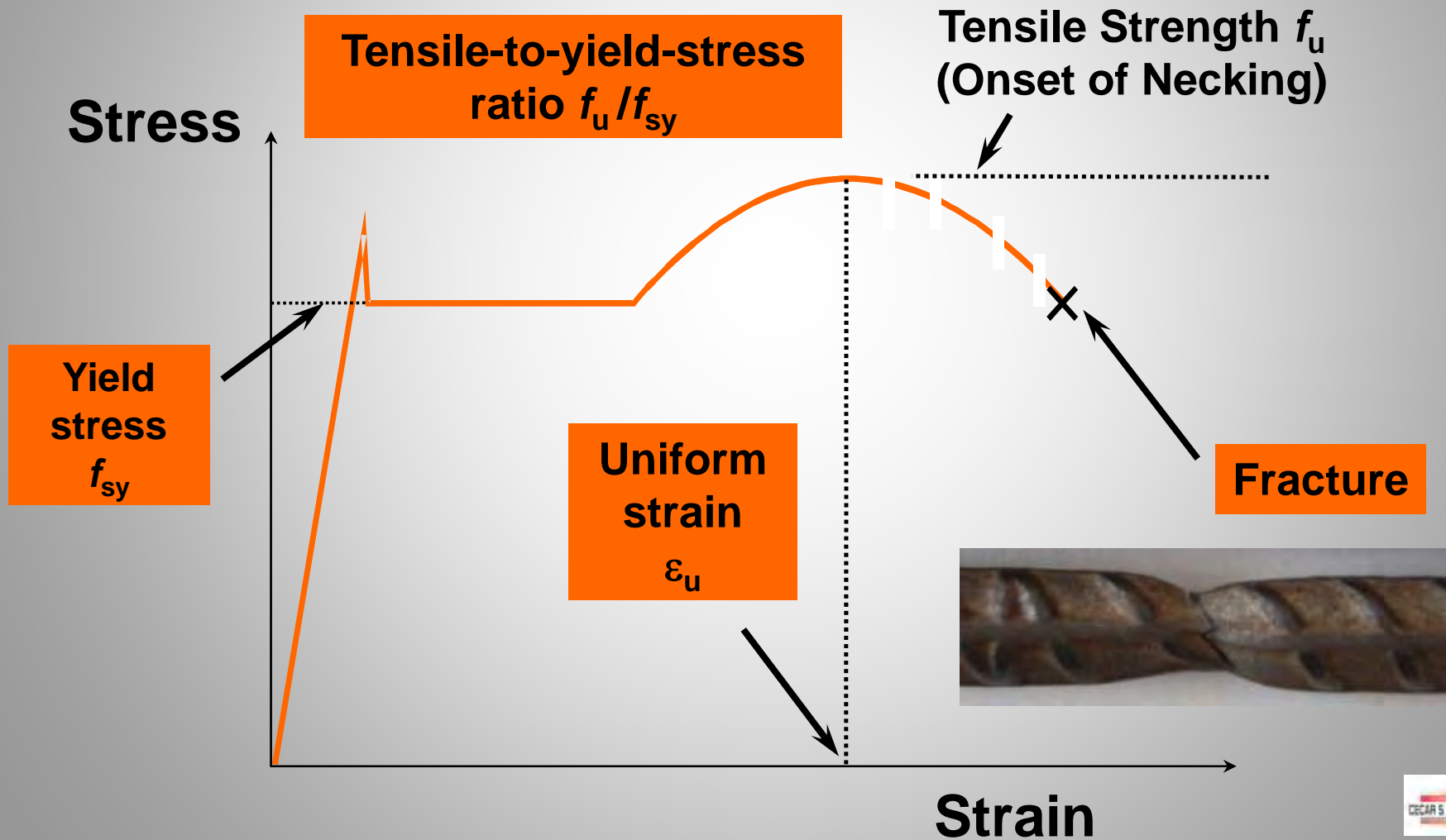
Overview

- **New Design Rules in AS 3600–2009**
 - **Straight D500N bars**
 - **Tensile development lengths**
 - **Tensile lap lengths**
- **Recent Australian Bond Test Series**
 - **University of New South Wales**
 - **University of Queensland**
 - **Curtin University of Technology (SRIA)**
- **Australian Bond Test Results**

Recent Australian Bond Test Results

New Design Rules in AS 3600–2009

- D500N Bars to AS/NZS 4671



Recent Australian Bond Test Results

New Design Rules in AS 3600–2009

- D500N Bars to AS/NZS 4671

<i>Property</i>	<i>D500N</i>
Nominal diameter (mm)	10 to 40
Characteristic yield stress (MPa)	
lower	500
upper	650
Tensile-to-yield-stress ratio, min.	1.08
Uniform strain (%) , min.	5.0

**DUCTILITY
PARAMETERS**

Estimate upper characteristic tensile strength = $1.15 \times 650 = 750 \text{ MPa} = 1.5f_{sy}$



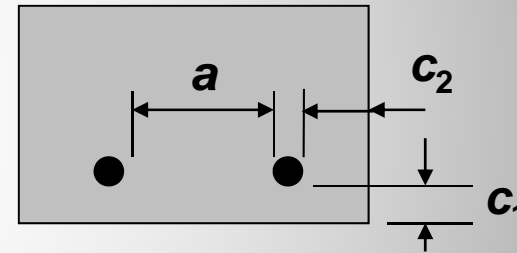
Recent Australian Bond Test Results

New Design Rules in AS 3600–2009

Basic Tensile Development Length:

$$L_{\text{sy.tb}} = \frac{50k_1 \left[(1.0 - 0.15(c_d - d_b) / d_b) \right] f_{\text{sy}} d_b}{(132 - d_b) \sqrt{f'_c}} \geq 29k_1 d_b$$

$$c_d = \min. (c_1, c_2, a/2) \\ \text{and } d_b \leq c_d \leq 3d_b$$



Refined Tensile Development Length:

$$L_{\text{sy.t}} = [1.0 - K(\sum A_{\text{tr}} - \sum A_{\text{tr.min}}) / A_s] [1.0 - 0.04\rho_p] L_{\text{sy.tb}} \geq (0.7/k_3) L_{\text{sy.tb}}$$

Transverse reinforcement
term, k_4

Transverse pressure
term, k_5

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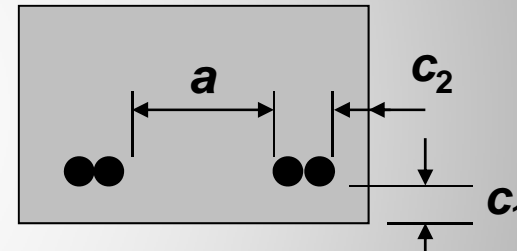
New Design Rules in AS 3600–2009

Basic Tensile Lap Length:

$$L_{\text{sy.tb.lap}} = k_7 \frac{50k_1 \left[(1.0 - 0.15(c_d - d_b) / d_b) \right] f_{\text{sy}} d_b}{(132 - d_b) \sqrt{f'_c}} \geq 29k_1 d_b$$

$$c_d = \min. (c_1, c_2, a/2)$$

$$\text{and } d_b \leq c_d \leq 3d_b$$



Refined Tensile Lap Length:

$$L_{\text{sy.t.lap}} = [1.0 - K(\sum A_{\text{tr}} - \sum A_{\text{tr.min}}) / A_s] [1.0 - 0.04\rho_p] L_{\text{sy.tb.lap}} \geq \max. [(0.7/k_3) L_{\text{sy.tb.lap}}, 29k_1 d_b]$$

Transverse reinforcement
term, k_4

Transverse pressure
term, k_5

Recent Australian Bond Test Results

New Design Rules in AS 3600–2009

What minimum tensile stress should anchored or spliced bars be capable of reaching before failure occurs?

*In design, it is assumed that the nominal yield stress, $f_{sy}=500$ MPa, will be reached before the anchorage or splice fails.....
but what about in the real structure?*

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New Design Rules in AS 3600–2009

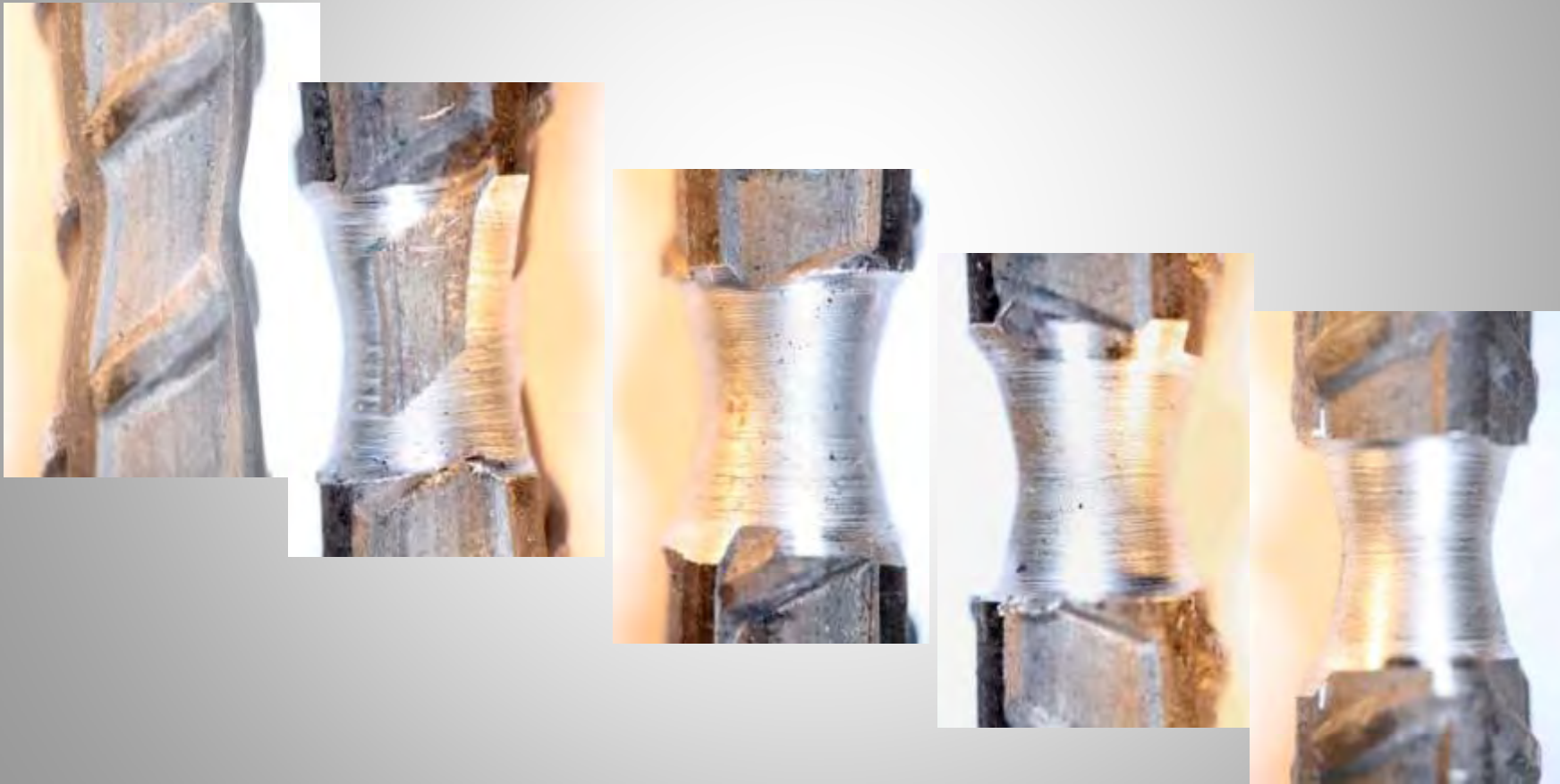
Clause 13.2.6 *Welded or mechanical splices*

“Welded or mechanical splices formed between Class N reinforcing bars should not fail prematurely in tension or compression before the reinforcing bars, unless it can be shown that the strength and ductility of the concrete member meet the design requirements.”

Recent Australian Bond Test Results

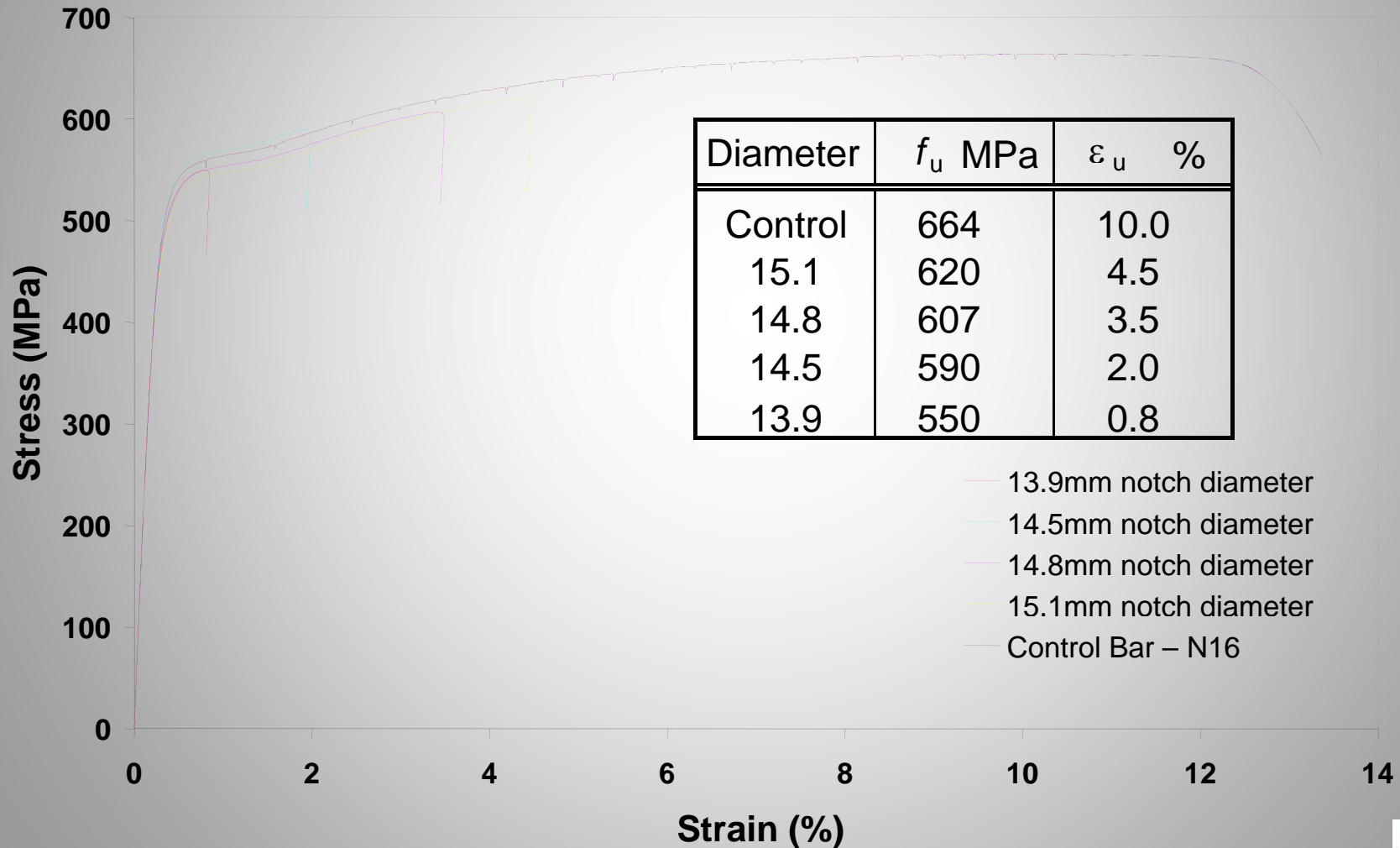
New Design Rules in AS 3600–2009

- **Any form of splice can reduce bar strength**



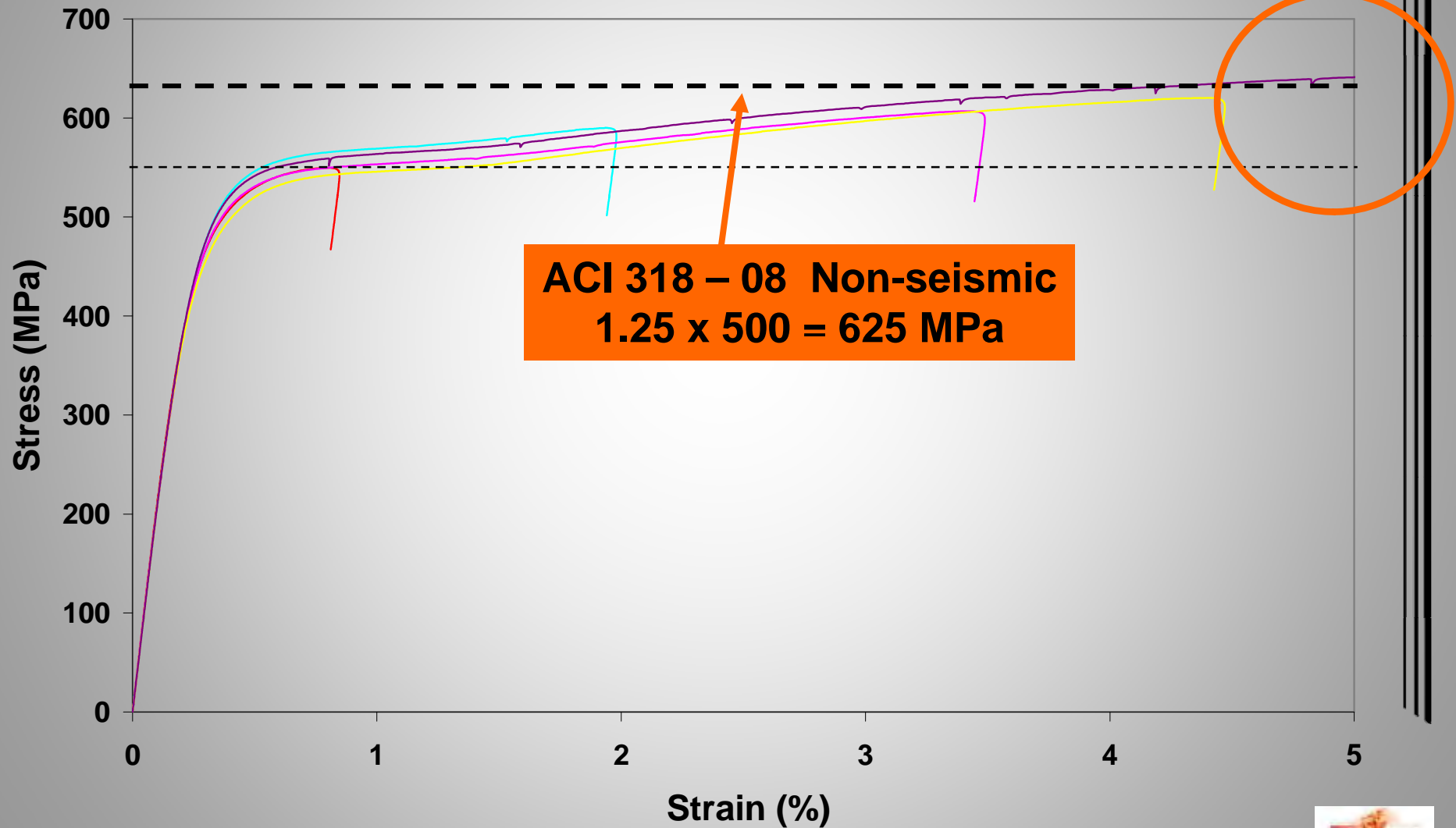
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New Design Rules in AS 3600–2009



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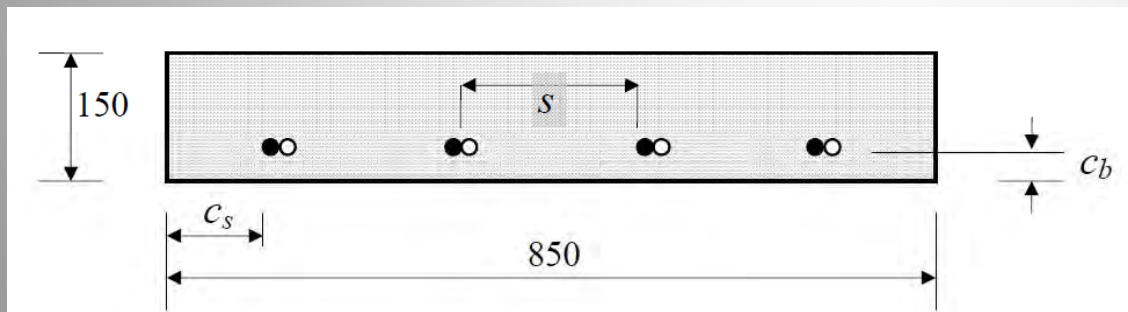
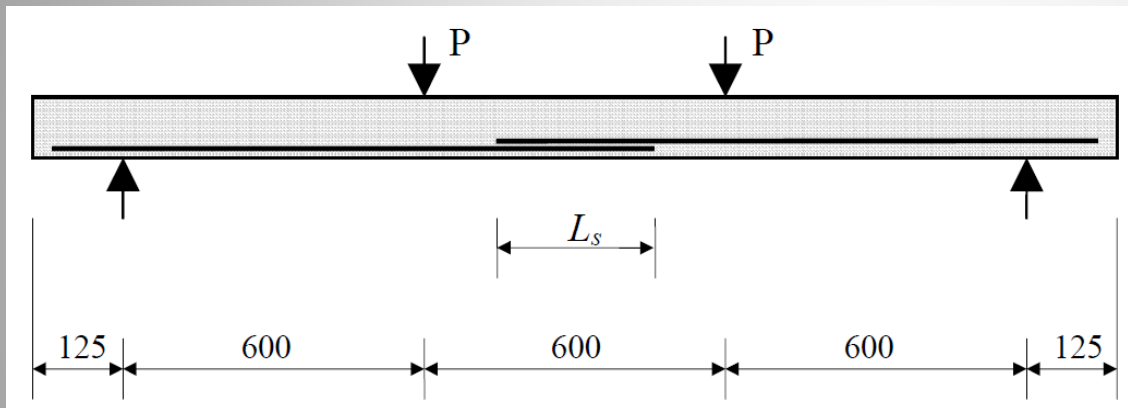
Design principles



Recent Australian Bond Test Results

Recent Australian Bond Test Series

■ University of New South Wales

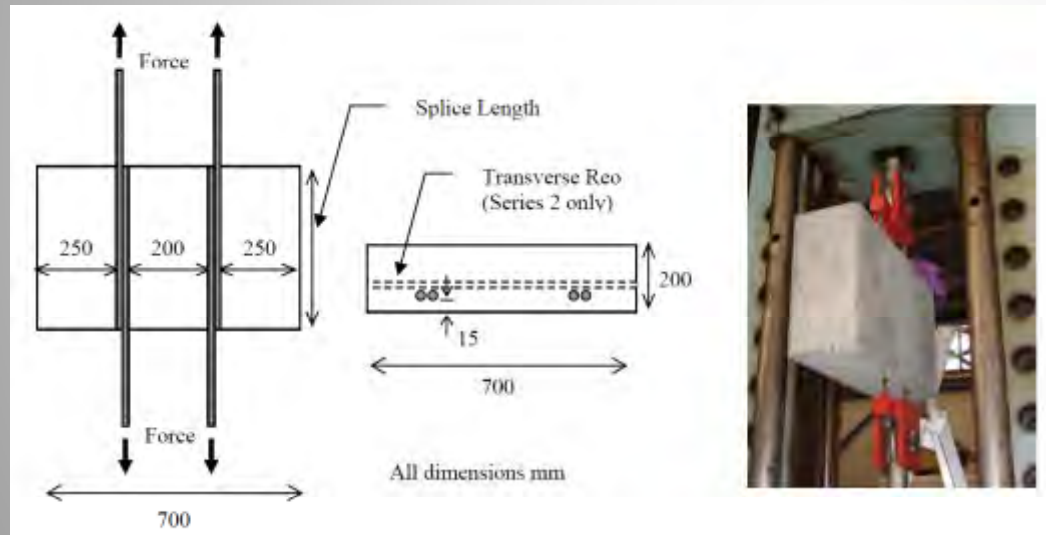


- 10 conventional slabs in flexure
- Small diameter, widely-spaced D500N12 or N16 bars representative of slabs or walls (min. clear distance 157 mm)
- No staggering
- No transverse reinforcement
- Short lap length (max. approx. $18d_b$) for bond failure

Recent Australian Bond Test Results

Recent Australian Bond Test Series

■ University of Queensland

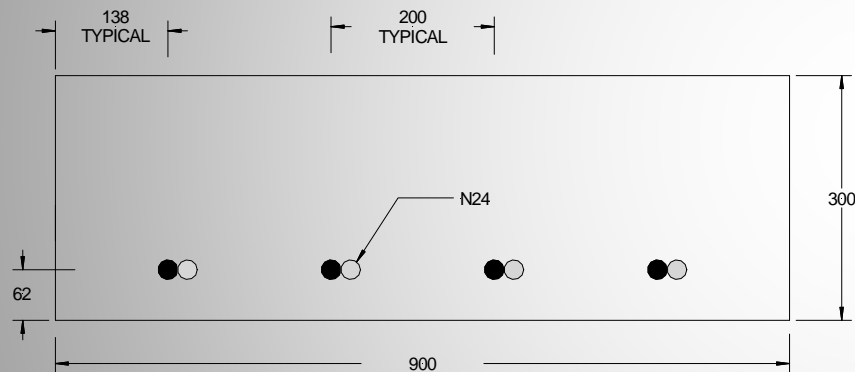


- Unconventional pull-out tests, without flexure to cause bar prying, etc.
- Small diameter, D500N16 bars representative of slabs or walls
- Contact lap splices
- No staggering
- Transverse bars present in 3 of these tests, but presence should be ignored in design since on wrong side of main bars

Recent Australian Bond Test Results

Recent Australian Bond Test Series

- Curtin University of Technology – SRIA tests



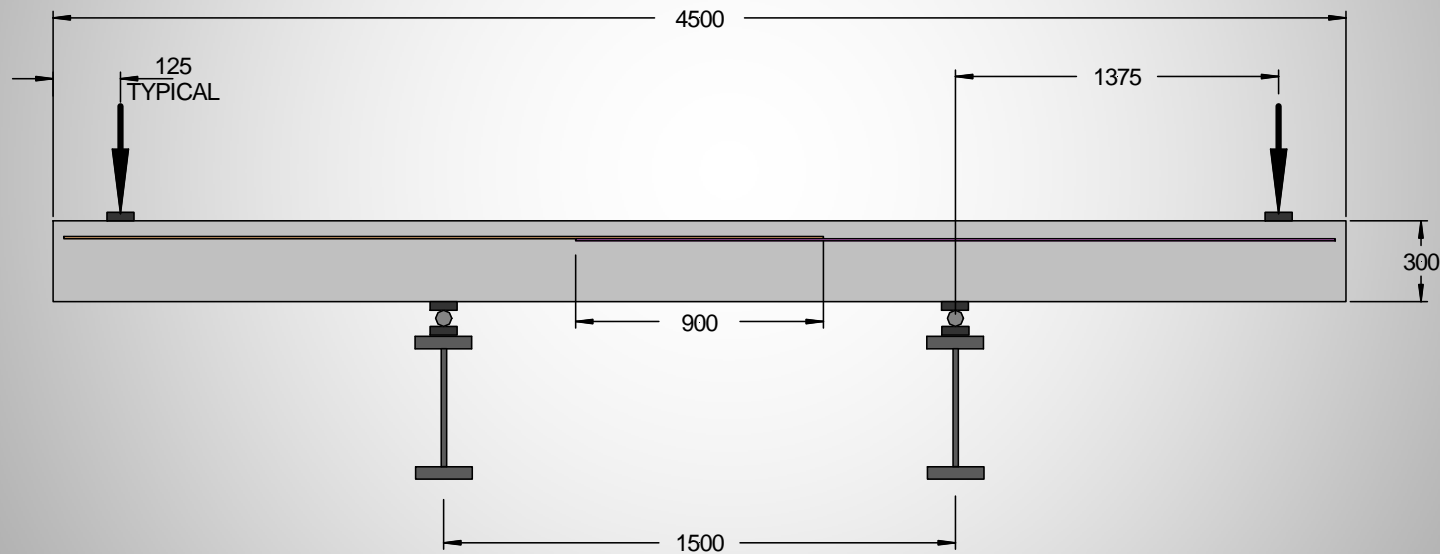
**4 N24 contact lap splices
without transverse reinforcement**



Recent Australian Bond Test Results

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Test lap length = 900 mm, while AS 3600–2009 design
 $L_{\text{sy.tb.lap}} = 930$ mm if use nominal f'_c and $k_7=1.0$

Recent Australian Bond Test Results

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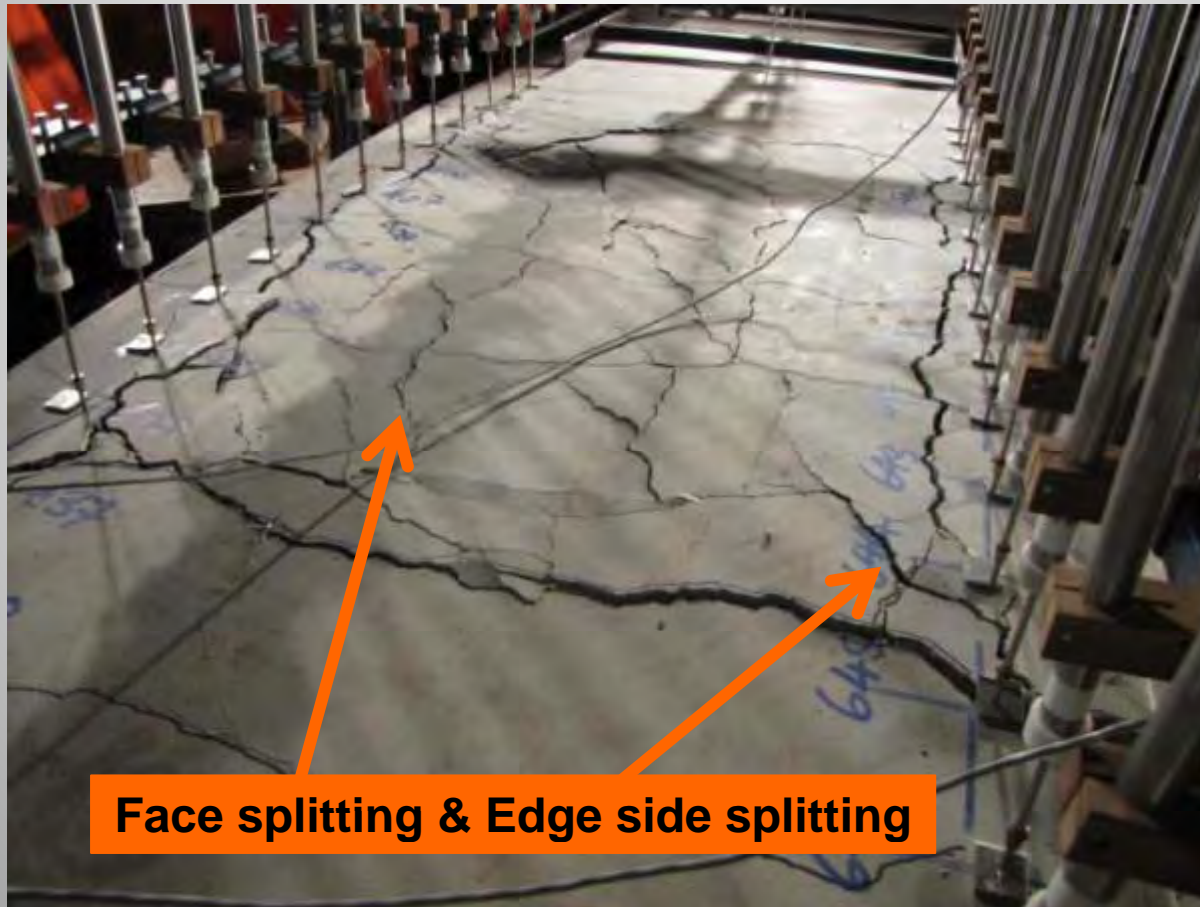
- **Curtin University of Technology – SRIA tests**



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Face splitting & Edge side splitting

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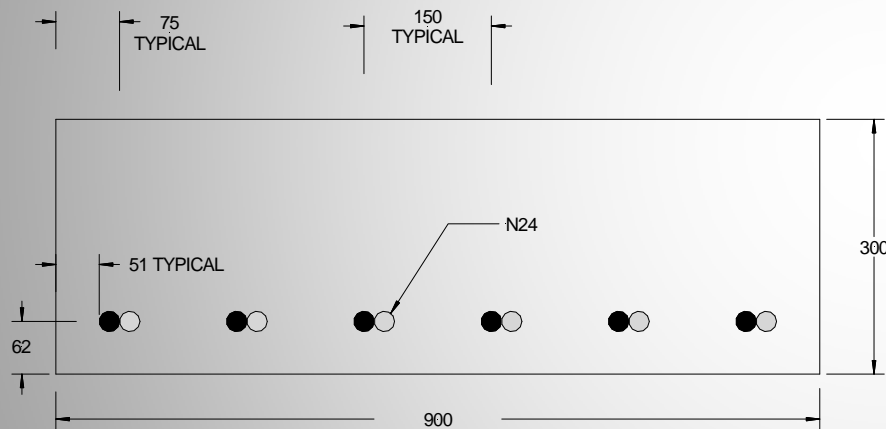
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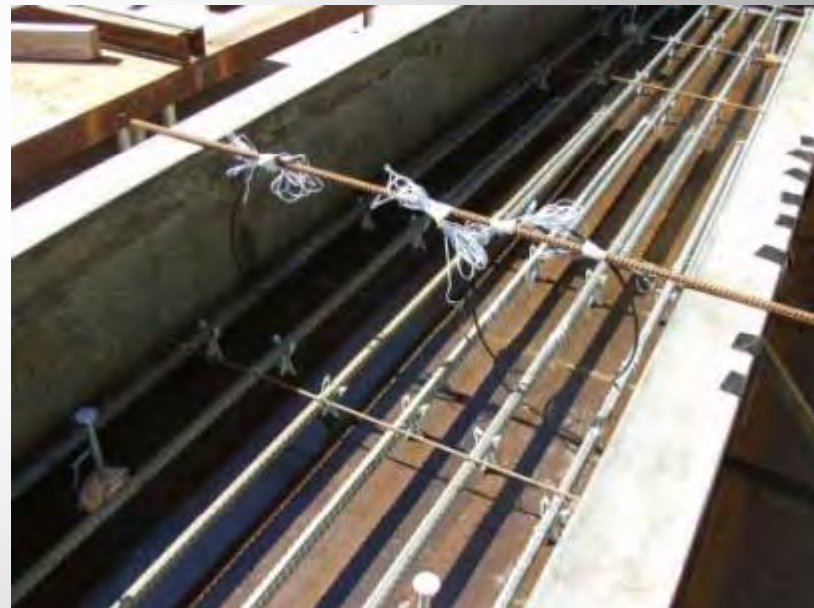
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**6 N24 contact lap splices
without transverse reinforcement**



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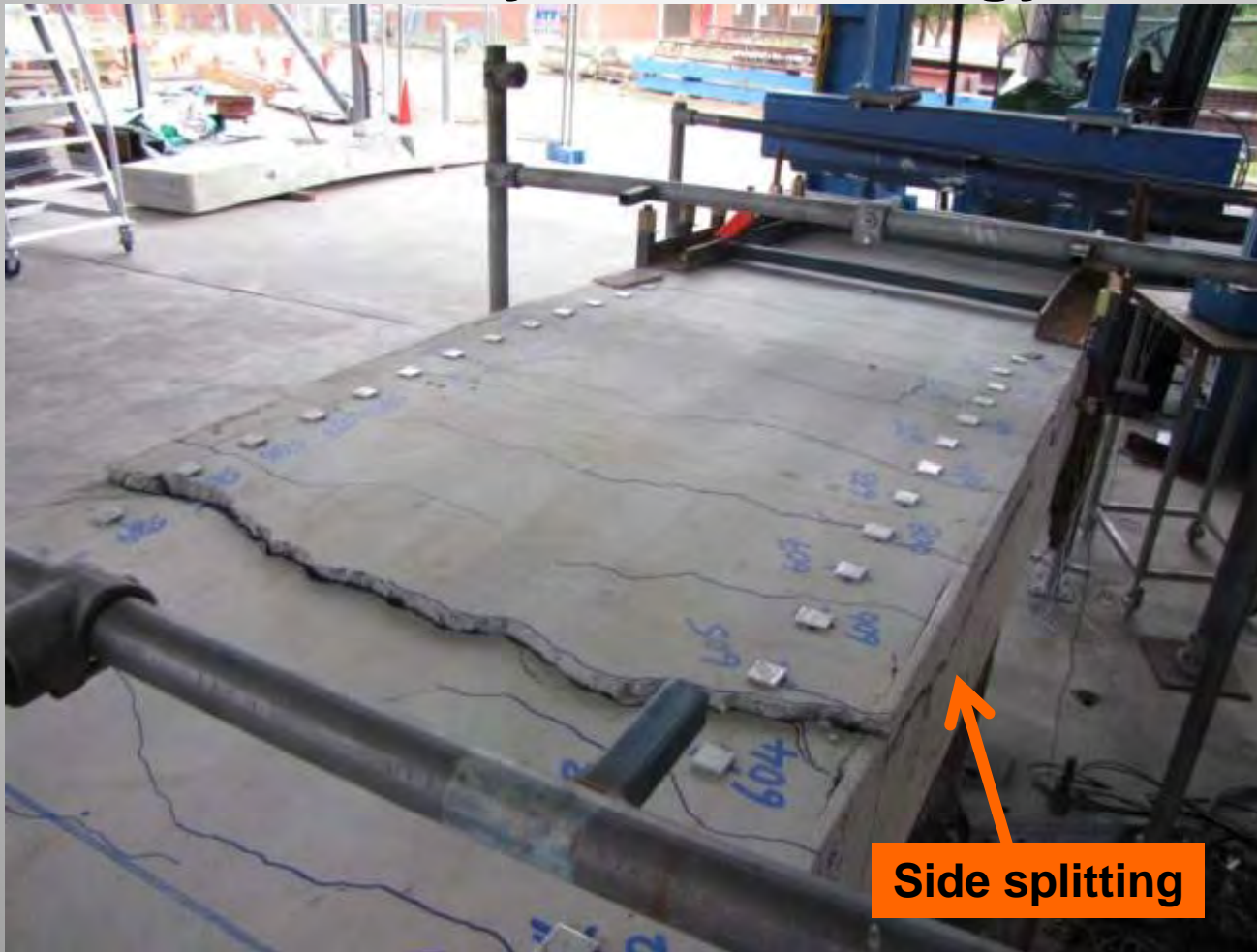
- **Curtin University of Technology – SRIA tests**



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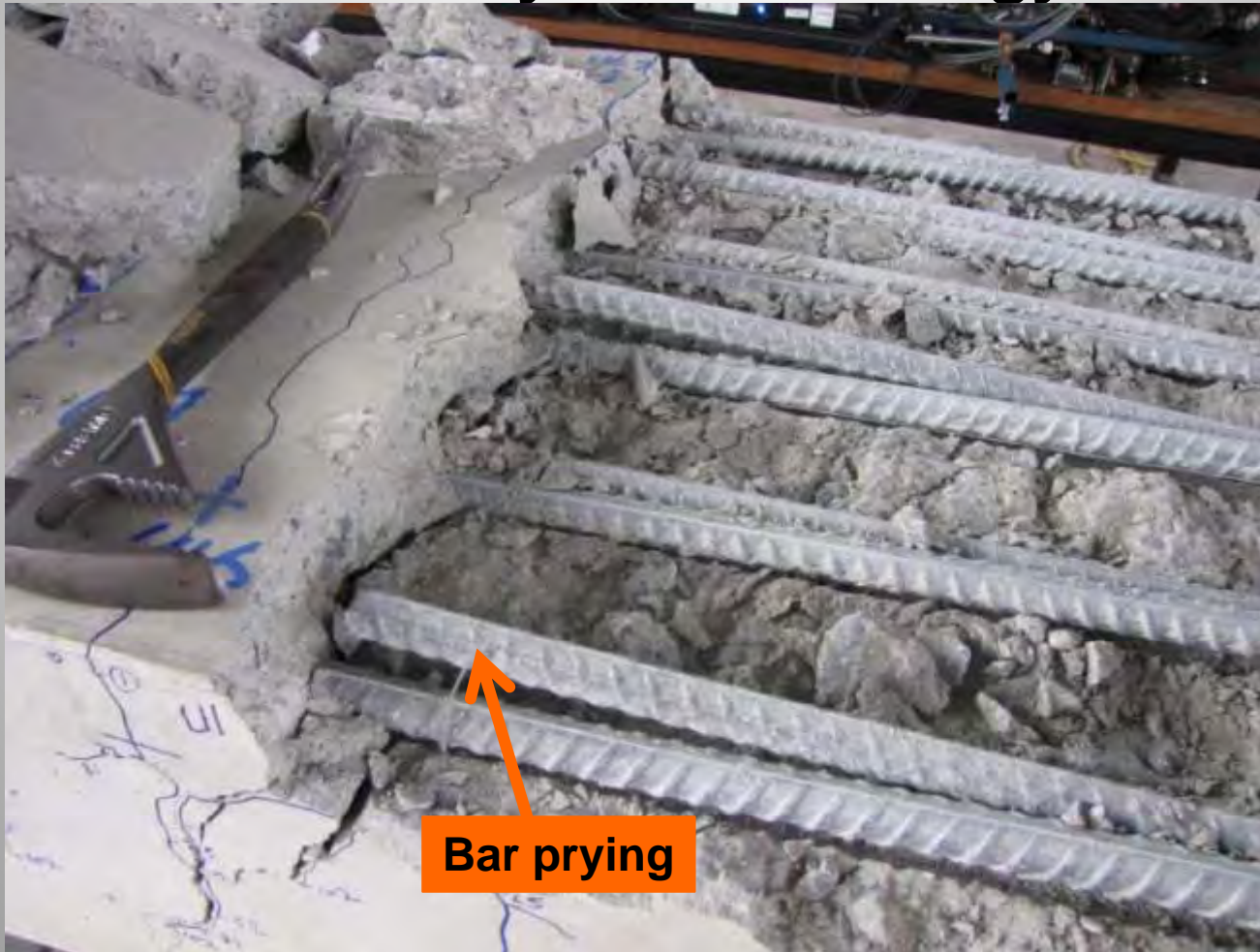
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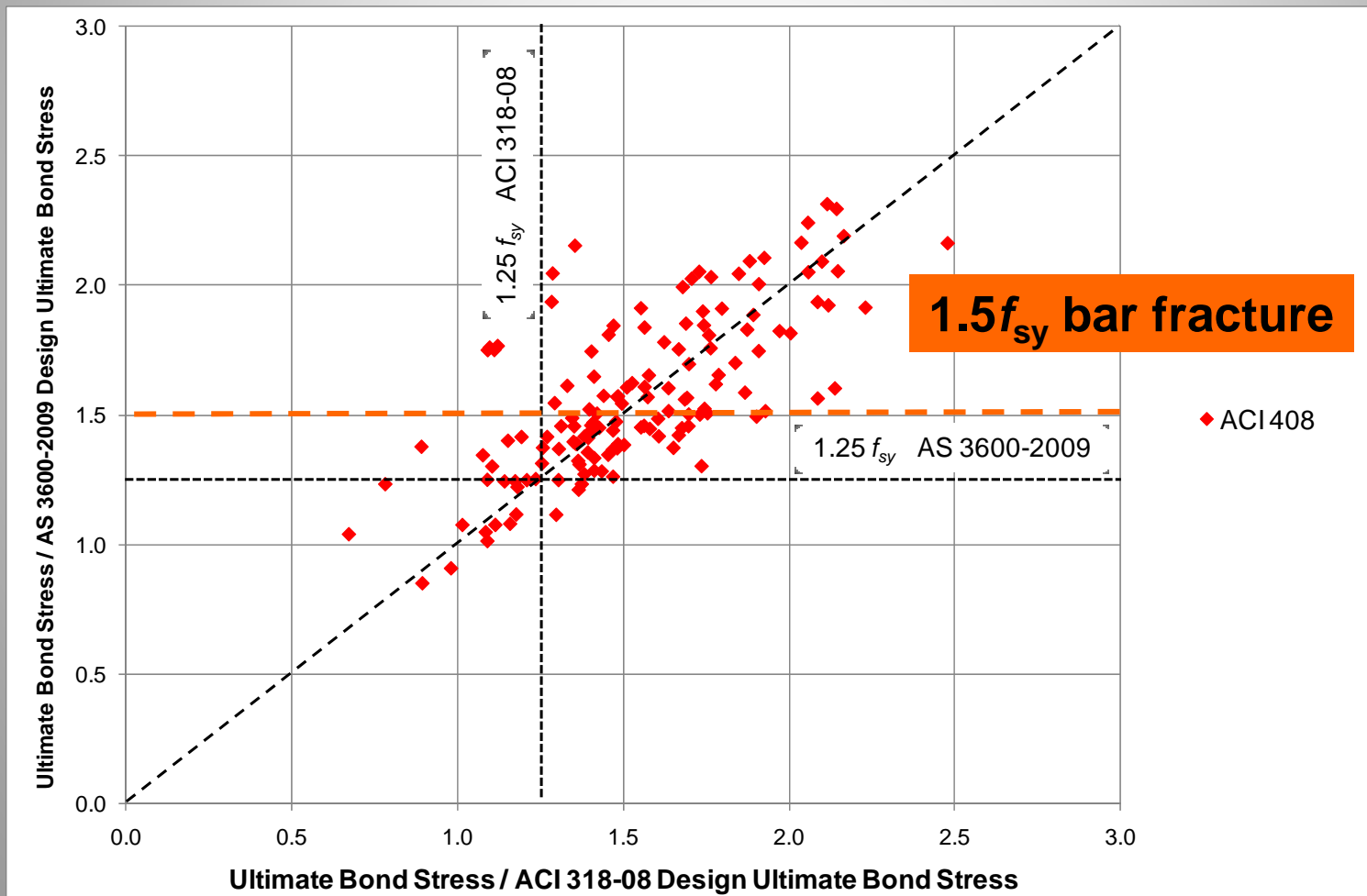
- **Curtin University of Technology – SRIA tests**



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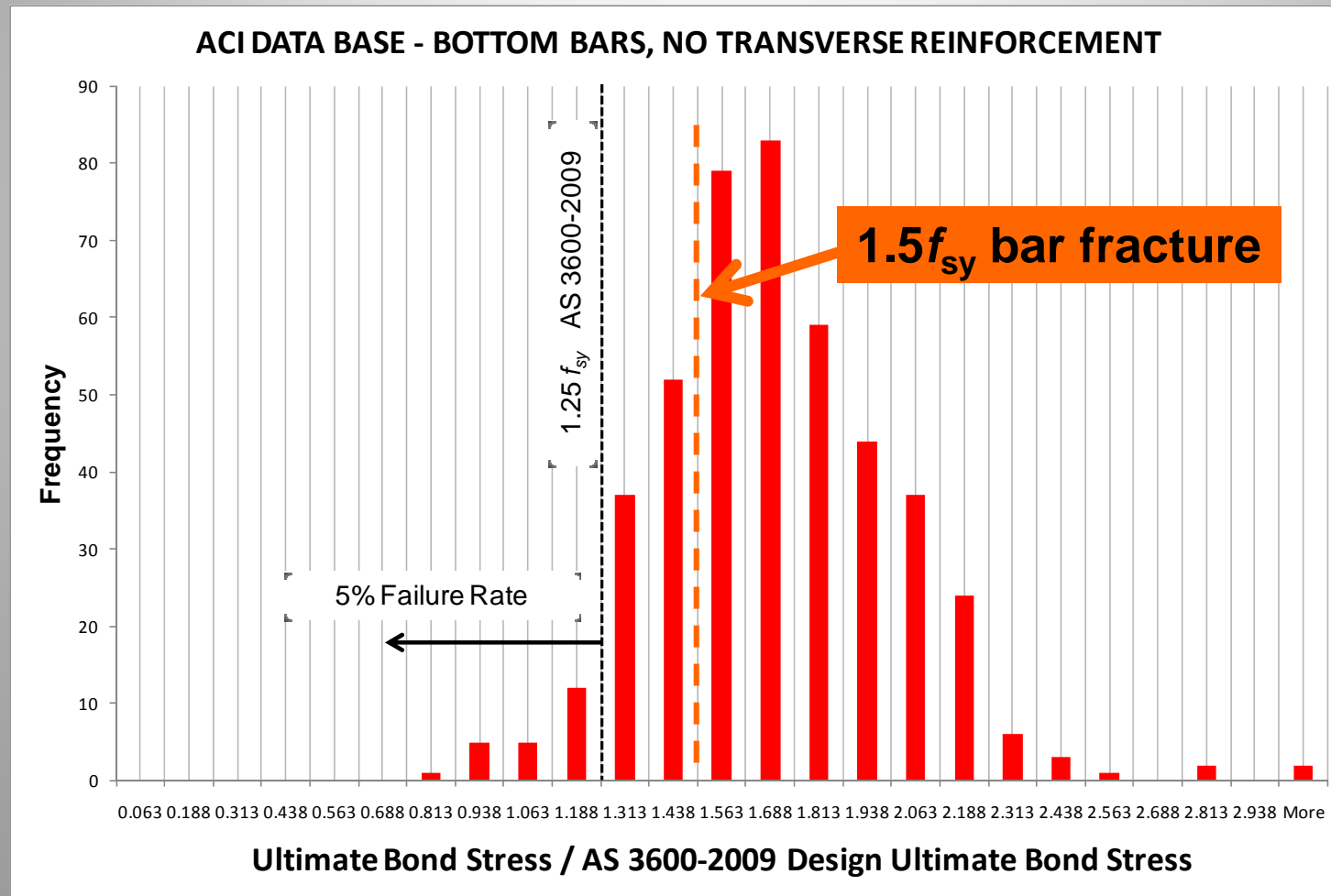
- American Concrete Institute (ACI) Database



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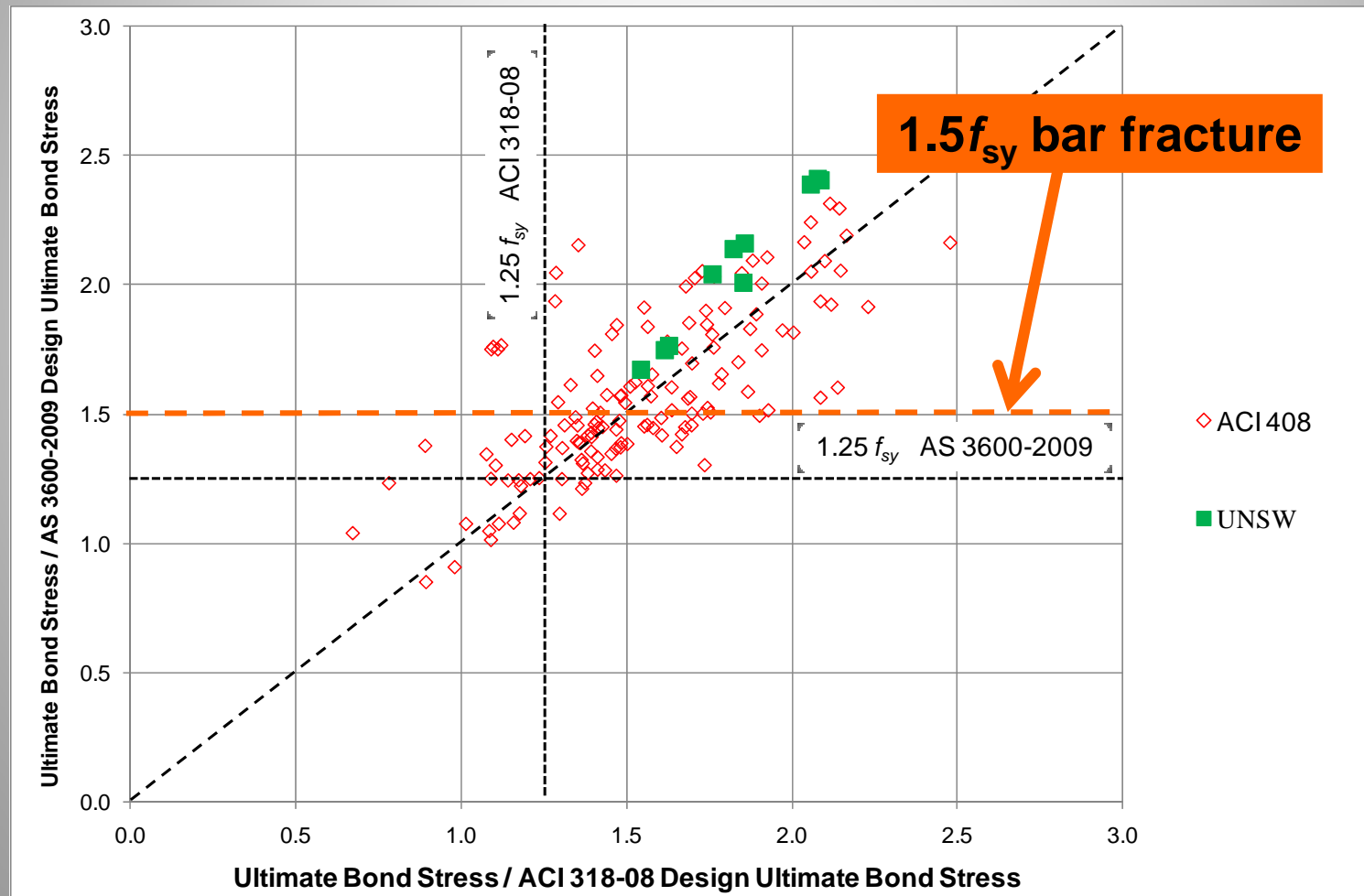
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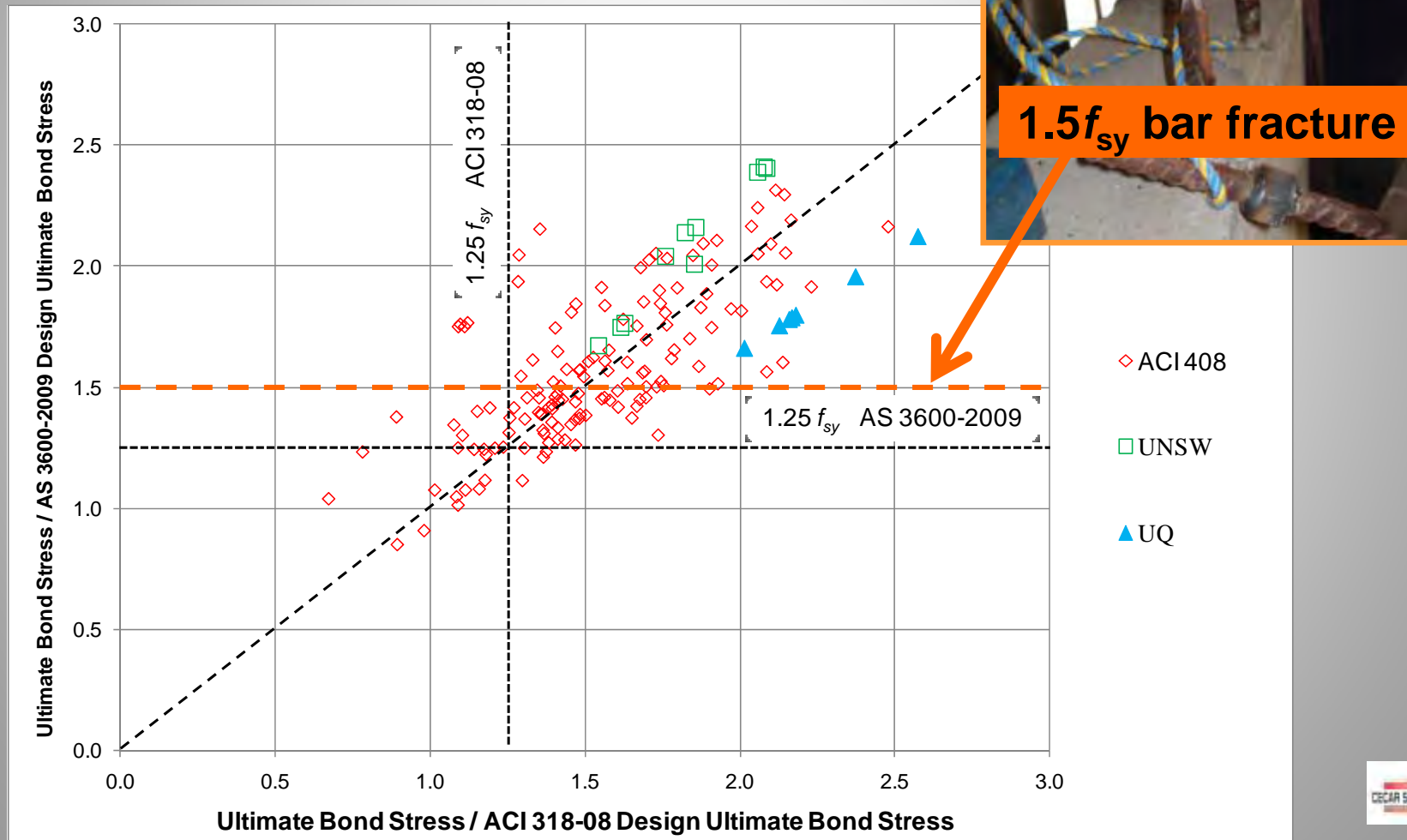
- University of New South Wales



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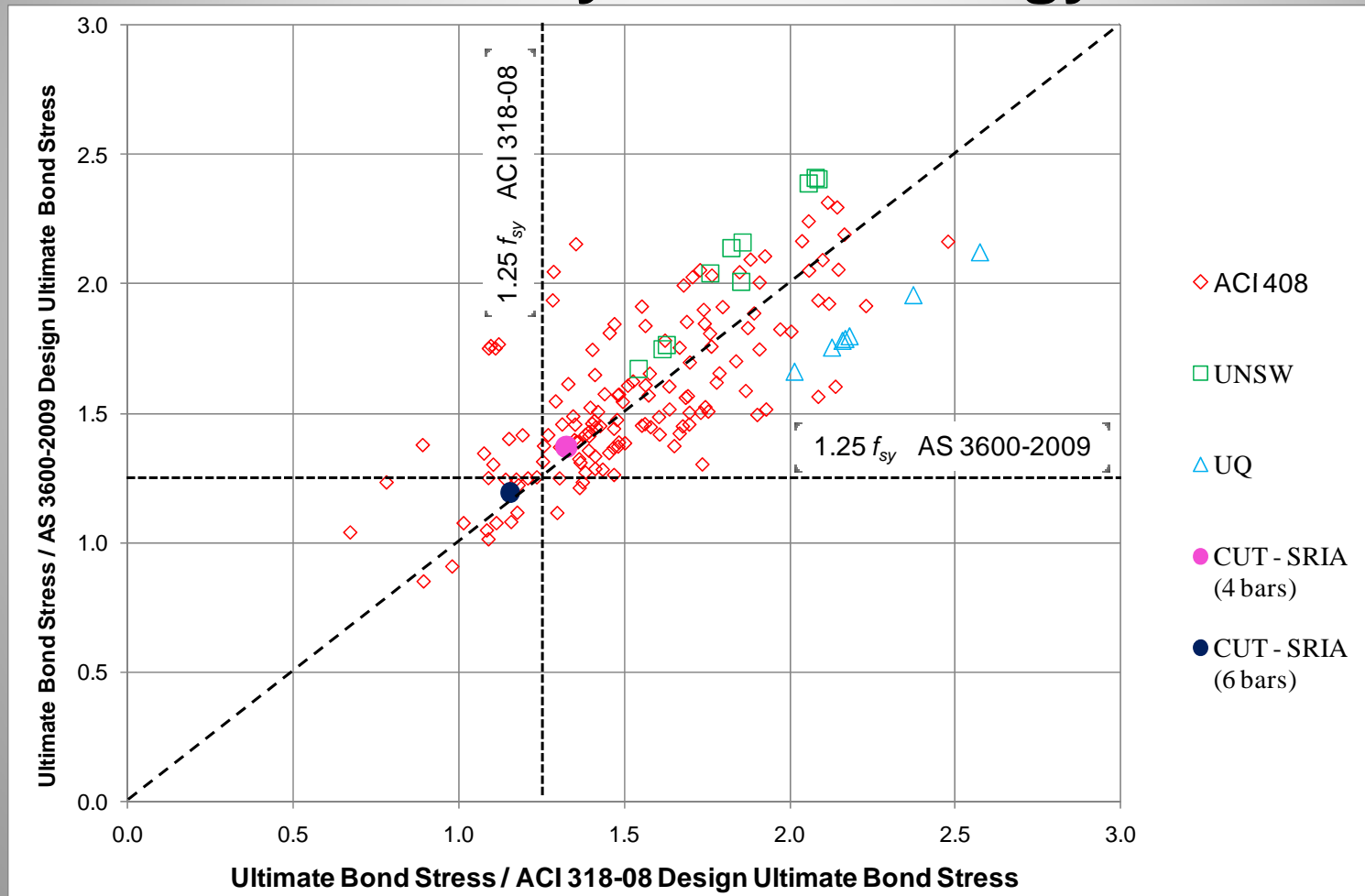
University of Queensland



Recent Australian Bond Test Results

Australian Bond Test Results

- Curtin University of Technology – SRIA tests



Recent Australian Bond Test Results

Bond Test Results & Conclusions

- UNSW & UQ tests were designed to replicate slabs & walls incorporating small diameter bars (D500N12 & 16)
- Results of UNSW and UQ tests on small diameter (N12 & N16) bars, with very high mean AS 3600–2009 test/design ratios of $2.1f_{sy}$ and $1.8f_{sy}$, respectively, indicate that **factor $k_2=(132-d_b)/100$ could possibly be increased**
- SRIA tests on beams with D500N24 bars performed by Curtin University also indicate **$1.25f_{sy}$ is more realistic**
- ACI database shows that **$1.25f_{sy}$ is a much more realistic target achieved using AS 3600–2009** under non-seismic conditions, which serves as a **ductility criterion** for spliced D500N bars generally



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